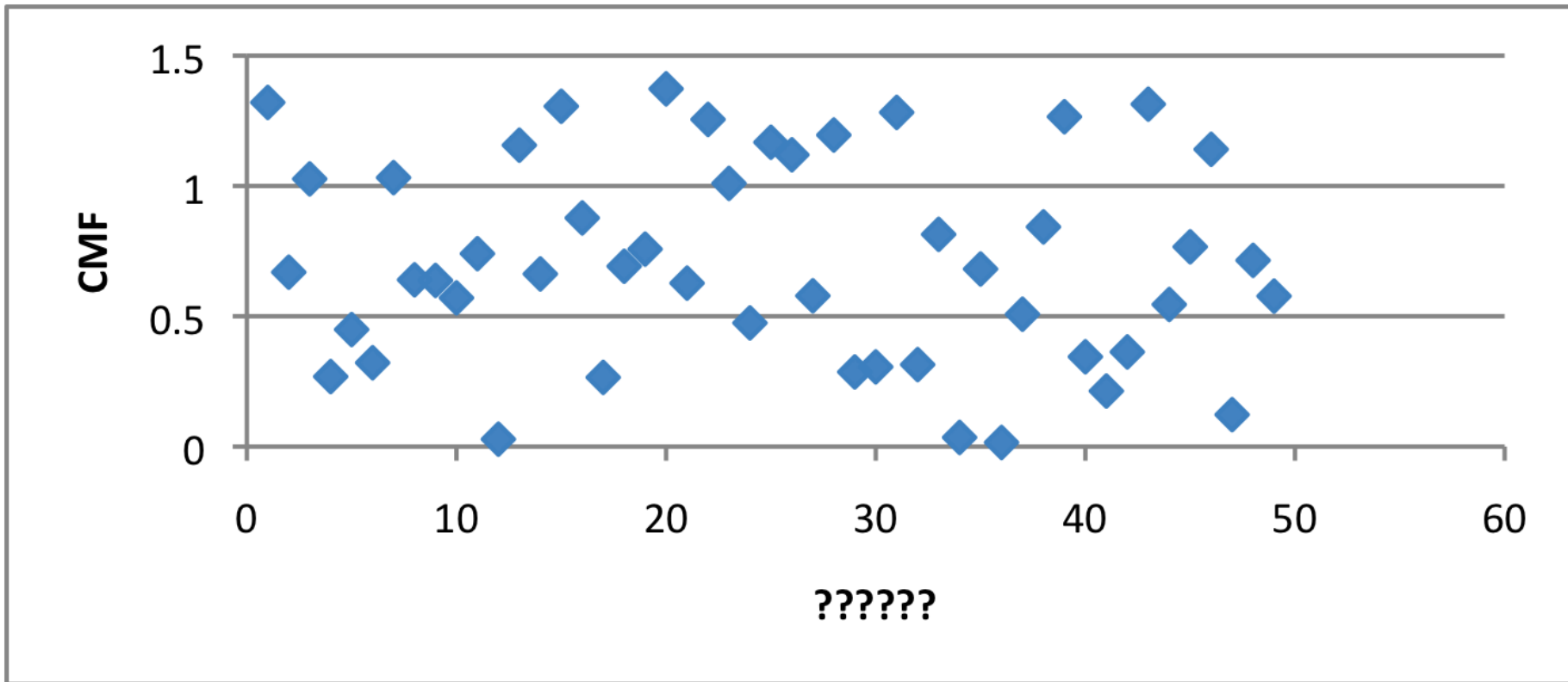


Variability in CMFs

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Who Works at a Workshop?

- A workshop is ... an educational seminar or series of meetings emphasizing interaction and exchange of information among a usually small number of participants.

.. The American Heritage Dictionary of the English Language

TOPICS (Focus is on before-after studies)

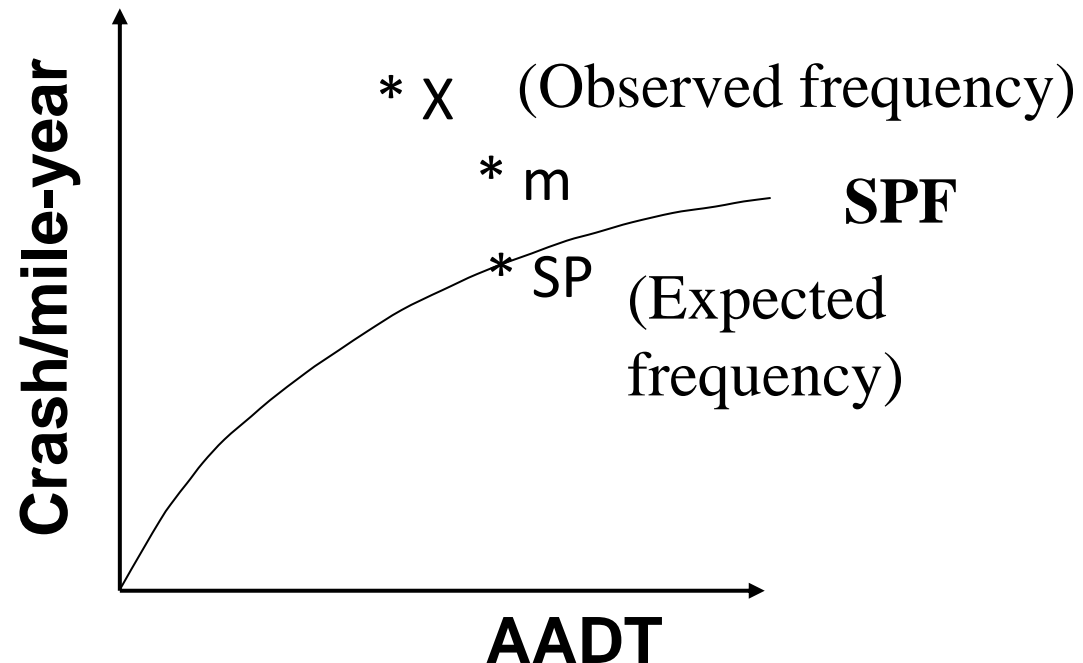
- a. Theoretical and Practical Issues in CMF development
 - Why different results can be obtained in different studies even with the best methods.
- b. Examples of variability in the recent CMF development.
- c. Accounting for variability in application

Theoretical and Practical Issues in CMF development from EB before-after studies

- Accounting for regression to the mean (RTM)
- Accounting for traffic volume changes
- Accounting for non-treatment effects
- Accounting for behavioural adaptation

Critical Elements of the Empirical Bayes Method

- Use safety performance functions to estimate expected (long term) accident frequency, m .
 - m is a weighted average of the safety performance function estimate (SP) and the observed accident frequency (X) of a site
 - Estimating SP is critical, especially if traffic volume or SPF changes or is sensitive to the choice of reference group used to estimate SPF
 - SPF should be used to account for non-treatment effects – HOW?



Discussion Points

- How important is the SPF in EB evaluations?
 - For RTM correction?
 - For accounting for AADT changes
 - For accounting for time trends
- How many EB studies use SPFs for all 3 purposes?
- What to do if SPFs are weak?

SIMPLE vs COMPLEX SPFs FOR EB STUDIES

(CMF depends on the Safety Performance Function used in EB analysis)

- Simple SPFs
 - Typically have AADT as the only independent variable
 - Crashes/year = α (AADT) ^{β}
 - Typically are estimated for different entity types and environments
 - 4 leg vs 3 leg
 - Urban vs rural
 - 2-lane, multilane, freeway
 - Typically are estimated with annual multipliers
- Complex SPFs

COMPLEX SPF

▶ Crashes that would have occurred without treatment

$$(\pi) = \alpha (\textit{Volume})^b (\textit{Speed})^c (\textit{Yellow})^d (\textit{Trucks})^e \\ (\textit{Through lanes})^f (\textit{Left lanes})^g$$

- ▶ where α = calibration parameter associated with a specific year
- ▶ b, c, d, e, f, g = calibration parameter associated with a geometric or operational variable
- ▶ *Volume* = ADT for the major road or the major plus minor road
- ▶ *Speed* = speed limit on major road in miles per hour
- ▶ *Yellow* = difference between yellow interval recommended by ITE and actual yellow interval
- ▶ *Trucks* = percentage of trucks in major road traffic stream
- ▶ *Through lanes* = number of lanes on major road approach
- ▶ *Left lanes* = total number of left-turn lanes from both major approaches.

CMF estimation must consider changes in AADT – Case of roundabouts

Accident type	Expected after without treatment		Recorded After	<i>Apparent</i> reduction	<i>Actual</i> reduction
	Naïve	EB			
All (with AADT change)	553	455	275	278 (50%)	180 (40%)
All (without AADT change)	436	354	275	161 (37%)	79 (22%)
Injury (with AADT change)	84	58	12	72 (86%)	46 (79%)
Injury (without AADT change)	68	48	12	56 (82%)	36 (75%)

Accounting for behavioral adaptation

- Problem of spillover
 - Effects underestimated if spillover sites used for comparison/reference group
 - E.g., red light cameras; speed control treatments; raised pavement markers
- Problem of migration
 - Effects underestimated if migration sites used for comparison/reference group
 - e.g., Speed control treatments; all-way stops

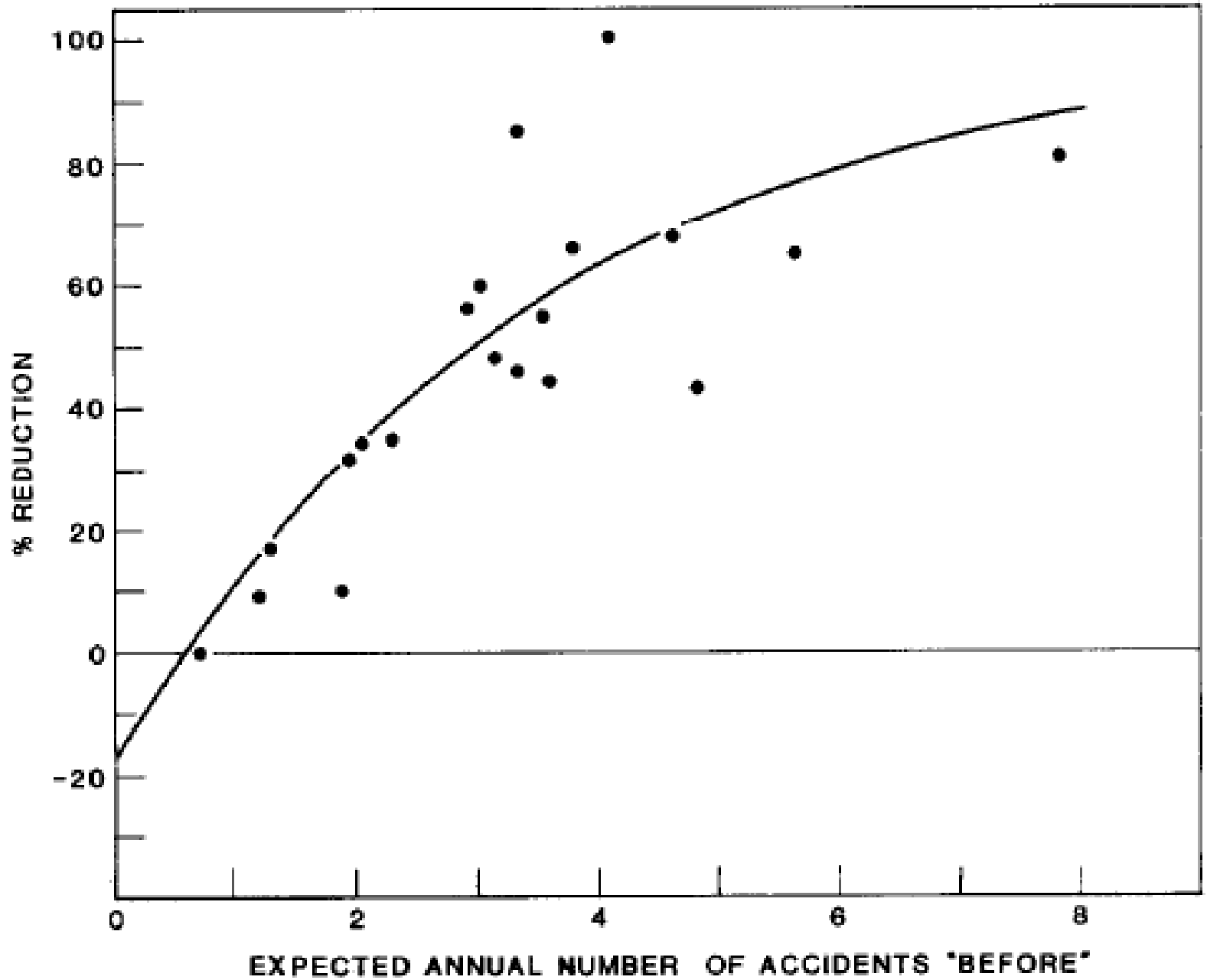
Discussion Points

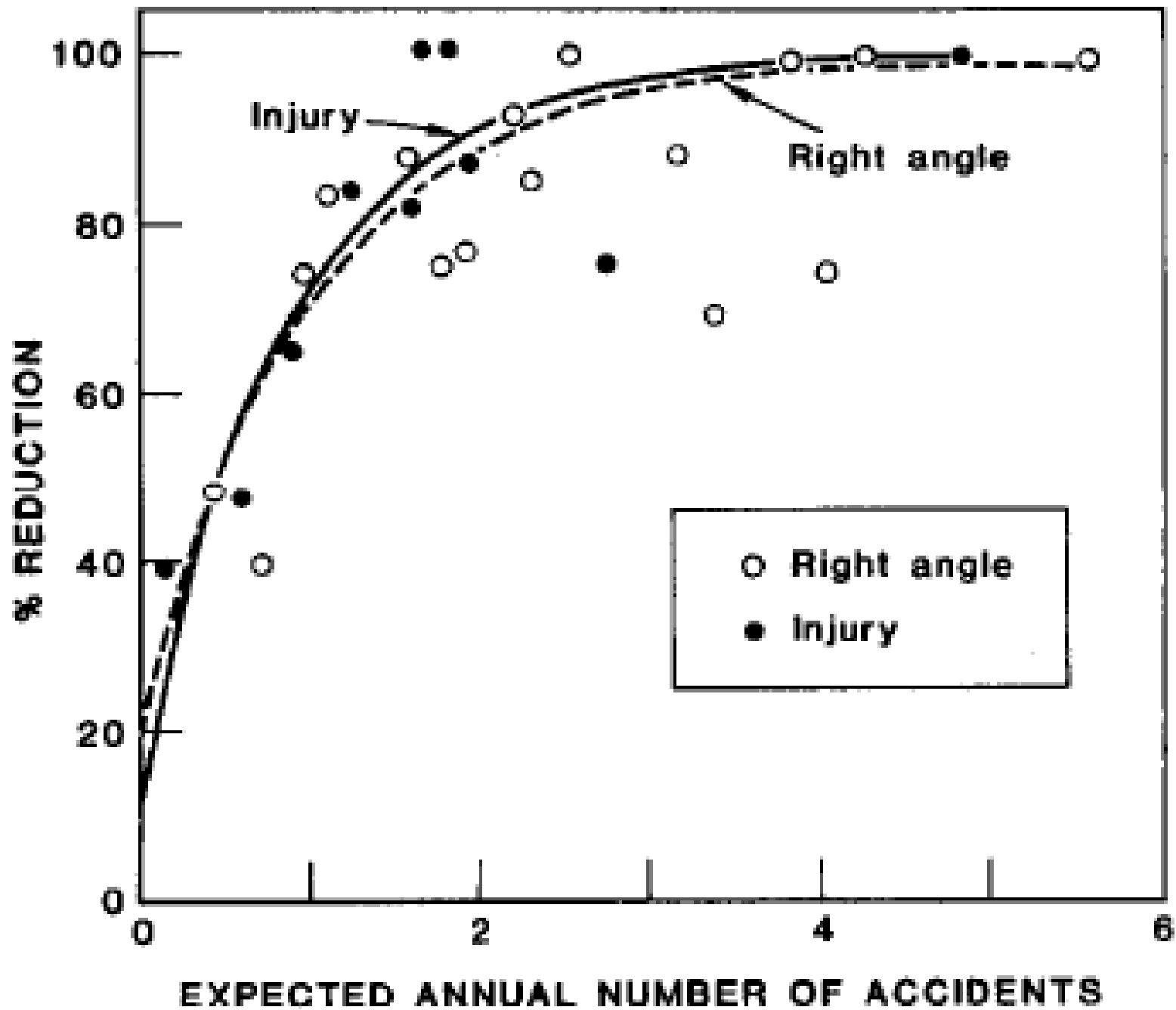
- What other treatments may be affected by spillover?
- What other treatments may be affected by migration?
- What existing CMFs may be affected?

Examples of Variability in Recent CMF Development

- Modifying the signal change interval
- Converting signals to roundabouts
- Offset lefts
- Two-way to all-way stop conversion
- Lane width/shoulder width
- Raised pavement markers

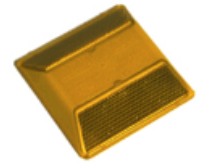
Conversion of intersections to all-way stop





Safety Evaluation of Raised Pavement Markers

- *Previous comparison group studies had found an increase in crashes*
 - *Used daytime crashes for comparison*
- % Reduction in crashes (EB Study)



	Night	Wet
New Jersey Non-Selective 174 miles	0.9	3.2
New York Selective 82 miles	12.7	20.2

Pavement markers: disaggregate effects (% reduction)

- Two Lane roads

	Flatter curves	Sharper curves
ADT <5000	-16%	-43%
5001-15000	No change	-26%
15001-20000	24%	-3%

- Freeways

	All sites
ADT <20,000	-13%
20001 to 60,000	6%
>60,000	33%

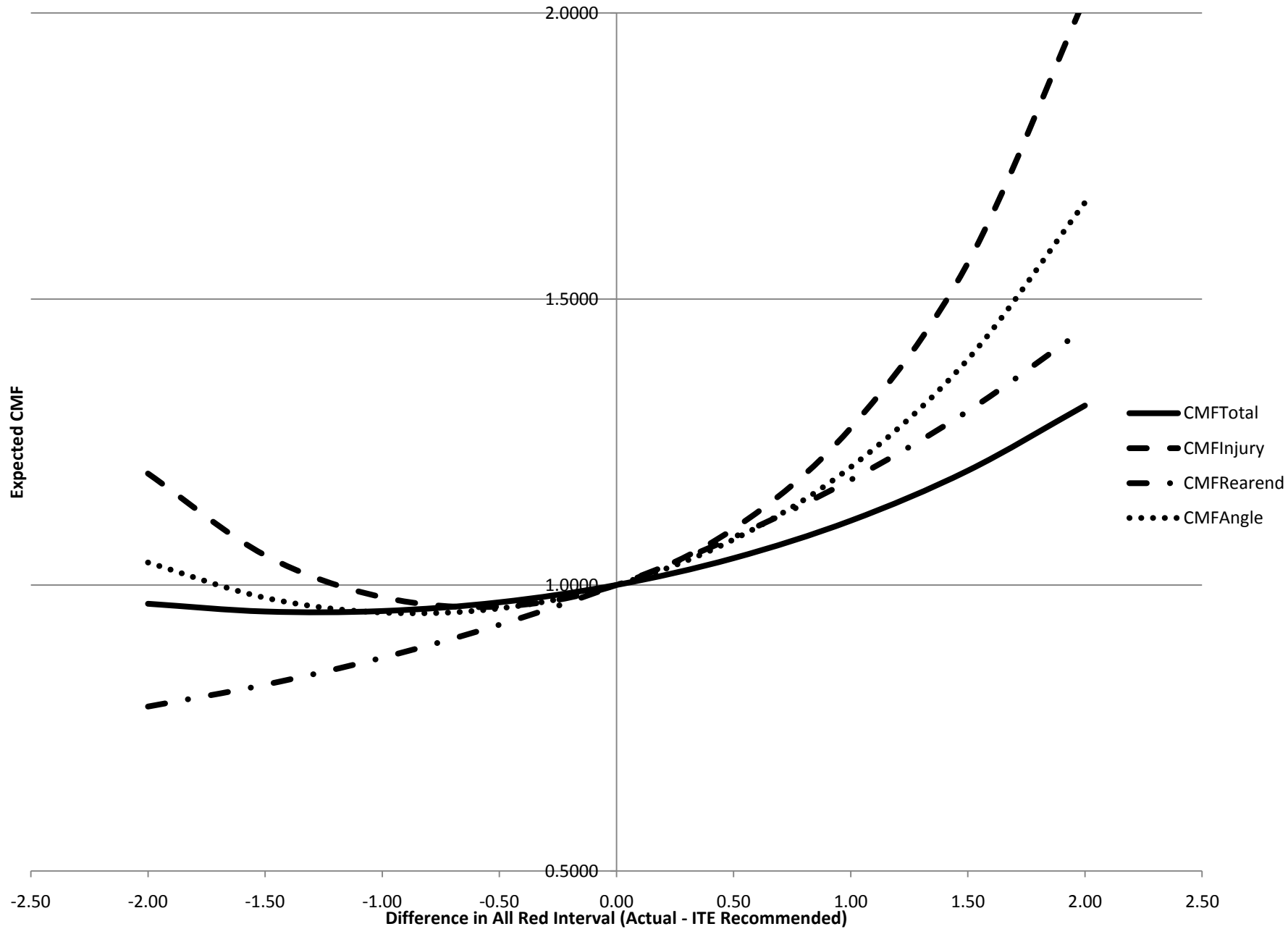
Improve skid resistance at targeted locations

- Previous research suggested that crashes could increase if drivers increase speed
- % reduction in crashes at locations with high skid numbers and wet weather accident frequency – applicability of CMF

Location Type	All Crashes	Wet-road	Rear-end Wet-road
Segments	23%	56%	43%
Intersection approaches	20%	57%	68%

Increase Signal Change Interval

Group	CMF for Total Crashes (SE)	CMF for Injury Crashes (SE)
Increase Change Interval (< ITE)	0.728 (0.077)	0.662 (0.099)
Increase Change Interval (> ITE)	0.922 (0.089)	0.937 (0.114)

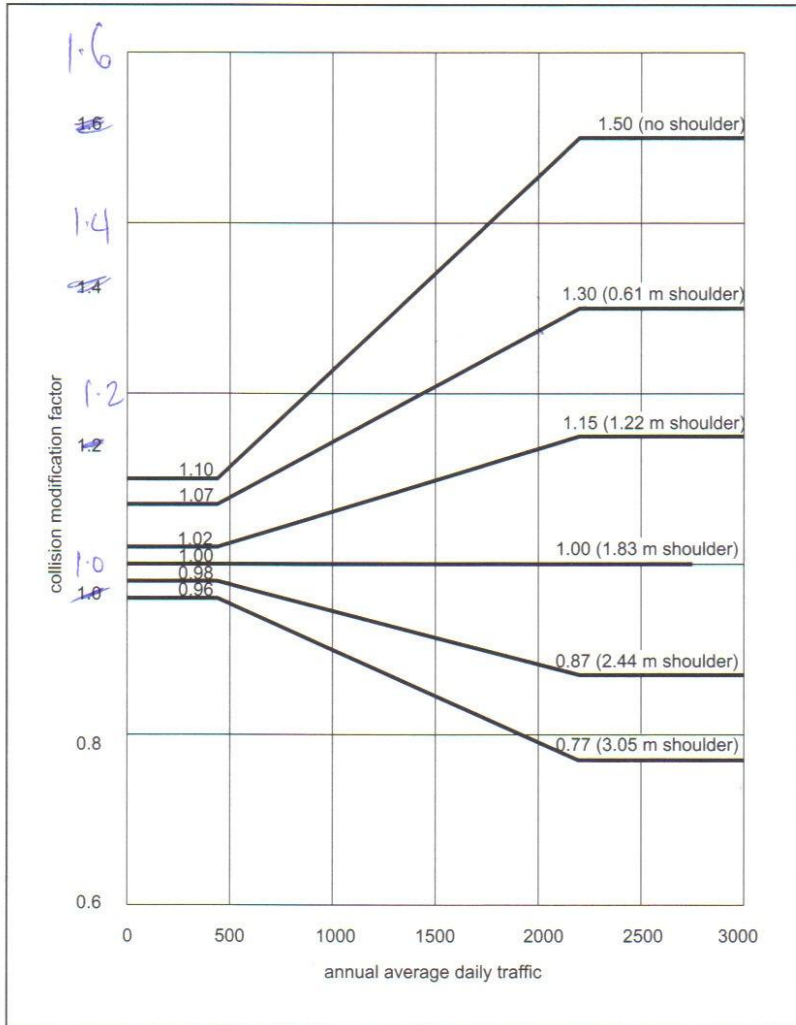


HSM CMFs for lane and shoulder width

Cross Section Elements



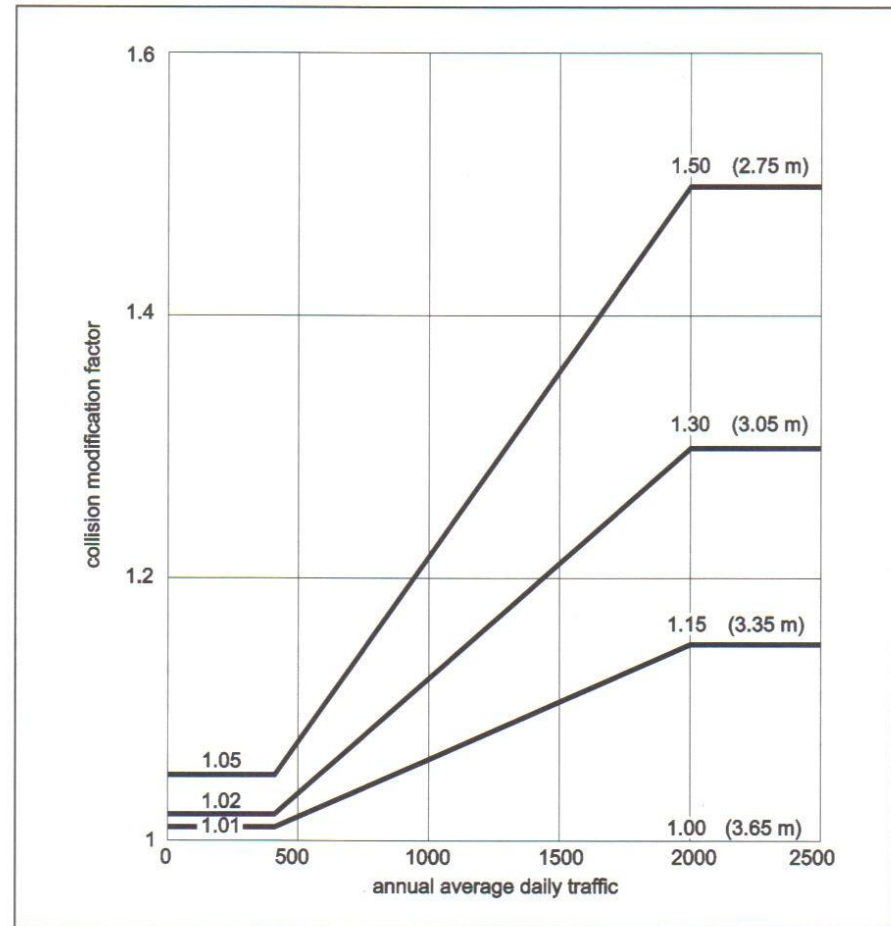
Figure 2.2.4.6 Collision Modification Factor for Various Shoulder Widths versus Annual Average Daily Traffic¹⁰



Cross Section Elements



Figure 2.2.2.1 Collision Modification Factor for Various Lane Widths and Traffic Levels versus Annual Average Daily Traffic³



CMFs for Lane-Shoulder Combinations on 2-lane roads

Pavement Width (ft)	10-ft Lanes	11-ft Lanes	12-ft Lanes
26	1.00	0.99	0.96
28	1.00	0.99	0.97
30	1.00	0.99	0.97
32	1.00	0.95	0.94
34	1.00	0.78	0.81
36	1.00	0.95	0.95

- ◆ CMFs for 4 ft shoulders are shaded
- ◆ Interaction between lane and shoulder width is important

% reduction in crashes from offset improvements for left-turn lanes

	Total	Injury	Left-Turn Opposing	Rear-End
Nebraska (Paint)	-0.5	6.2	-45.0	-6.9
Wisconsin Major construction)	33.8	35.6	38.0	31.7

Note: A negative sign indicates an increase in crashes.

- If > 9 crashes/year Nebraska installations have 8% reduction and are highly cost-effective!!
- CMF depends on intensity of treatment and on whether there is a crash problem

Economic Analysis (Nebraska)

- **COST**
 - \$200 per approach (8-10 year service life);
- **BENEFIT:COST**
 - Reduction of 0.64 crashes/year for 2:1 benefit cost ratio
 - Achievable if 9 or more crashes expected per year, for which the crash reduction factor is at least 8%

Accounting for Variation in CMFs

- Major Design Change
- Minor improvements

Accounting for CMF Variation in Estimating Safety Consequence of Major Design Changes

- E.g.
 - Roundabouts
 - New FHWA Roundabout Guide
 - Recent MTO/Transport Canada Research
 - Traffic Signals
 - NCHRP491
 - Adding lanes to segment
 - Add lanes to intersection
- Method:
 - Estimate (I) expected crashes without change (Use EB Method)
 - Estimate (II) expected crashes with change
 - Use a Safety Performance Function for facility with the change
 - Compare (i) and (II)

Estimating Safety Without Major Design change

- Account for variation in applications – need to always do disaggregate analysis in EB studies
 - Research needs are different from agency needs

- Use CMFunctions

Recent example: Conversion of signals to roundabouts:

$$\text{CMFunction} = (4\text{E}-05) * \text{AADT} + 0.303$$

DiscussionPoints

- What are the implications for international transferability of CMFs?
- What messages can we take to the afternoon session

Summary → Segue

- CMF Variability impacts transferability
- CMF variability can be due to application circumstance
 - Level/intensity of application
 - Level of existing safety problem
 - Traffic, geometry, other
- CMF variability can be due to methodological issues – even if EB is used
 - Quality of reference group used to develop SPFs
 - Quality of SPF development
 - How AADT and other non-treatment effects are accounted for